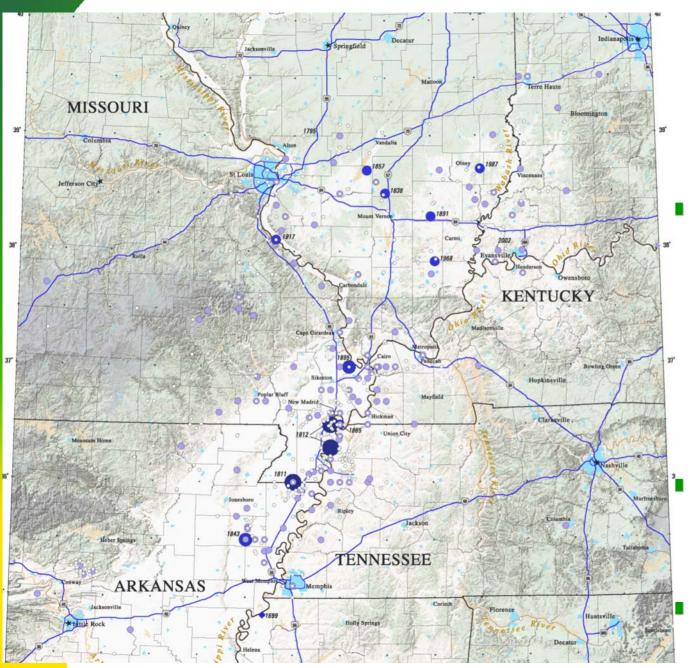
# BRIEF OVERVIEW OF SEISMIC THREAT POSED TO STRUCTURES IN THE ST. LOUIS METRO AREA

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MIDWEST SEISMIC SOURCES

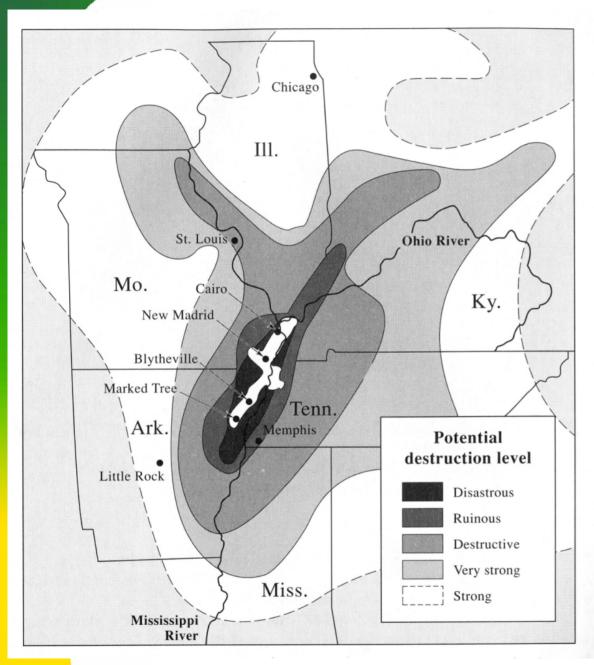
Not all of the region's quakes emanate from the New Madrid **Seismic Zone** Wabash Valley **Seismic Zone South Central** Illinois

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# POST 1812 SEISMICITY in NEW MADRID SEISMIC ZONE

- M6.3 quake in Marked Tree, AR in 1843; did considerable damage to Memphis, 60-70 km east
- M6.6 quake in Charleston, MO in 1895; Felt in 23 states, 30 km of sand blows
- M5.4 in Wabash Valley (Dale, IL) in 1968; also felt in 23 states; light damage in St. Louis
- M5.0 in Wabash Valley west of Vincennes, IN (Olney, IL) in 1987
- M4.6 near Evansville, IN in 2002

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## DAMAGE POTENTIAL

Published damage predictions for the New Madrid Seismic Zone have focused on the near field area, in the upper Mississippi Valley

These are based on synthetic motion time histories with assumed soil cover; not on site specific characteristics or dynamic properties of structures.

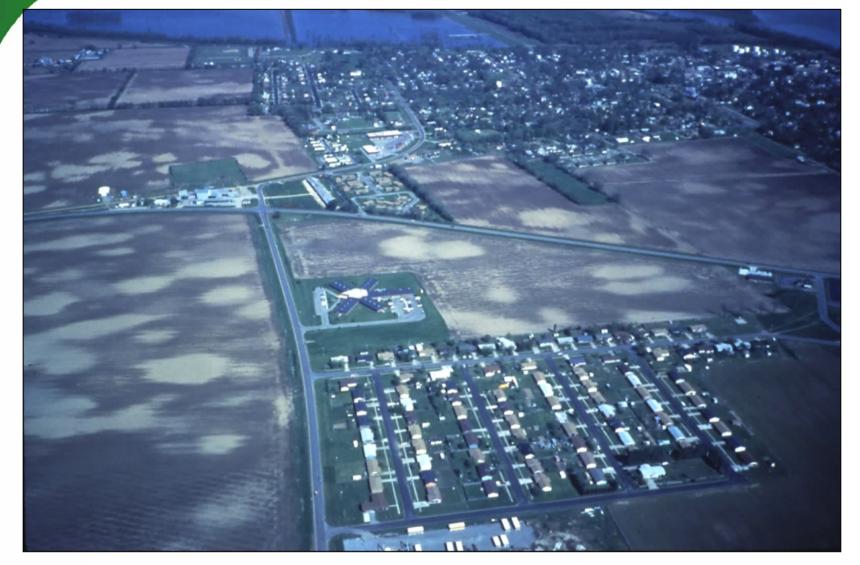




## LIQUEFACTION or "QUICK SAND"

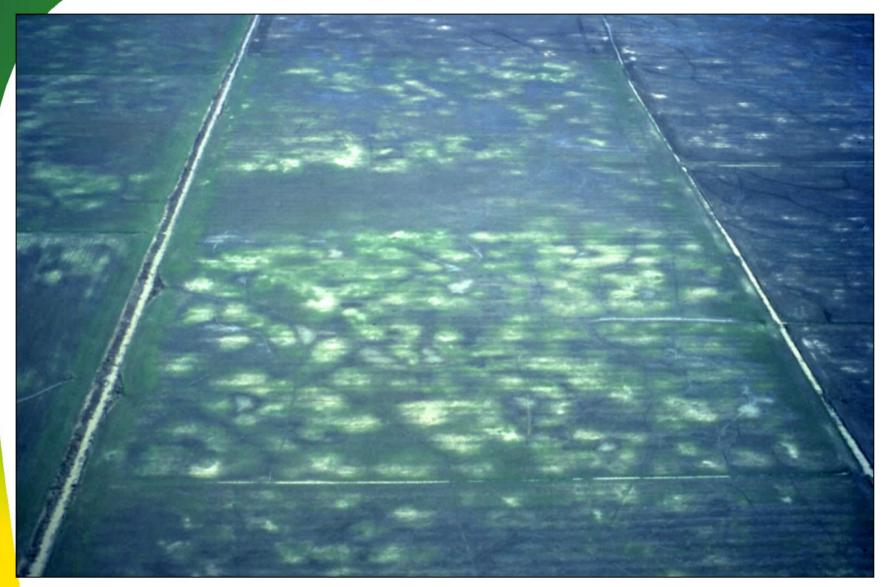
Liquefaction is a failure mechanism by which cohesionless materials (sand and silt) lose shear strength when the pore pressure is excited to a level equal to the effective confining stress. Usually limited to the upper 50 feet and typically occurs in silt, sand and fine gravel.





Recent sand blows dot the landscape surrounding New Madrid, MO, testifying to massive liquefaction

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### Enormous tracts of land exhibit evidence of paleoliquefaction – on a grandiose scale





Farm lands west of Big Lake, AR reveal a series of linear fissures which disgorged liquefied sand from beneath a silt cover.

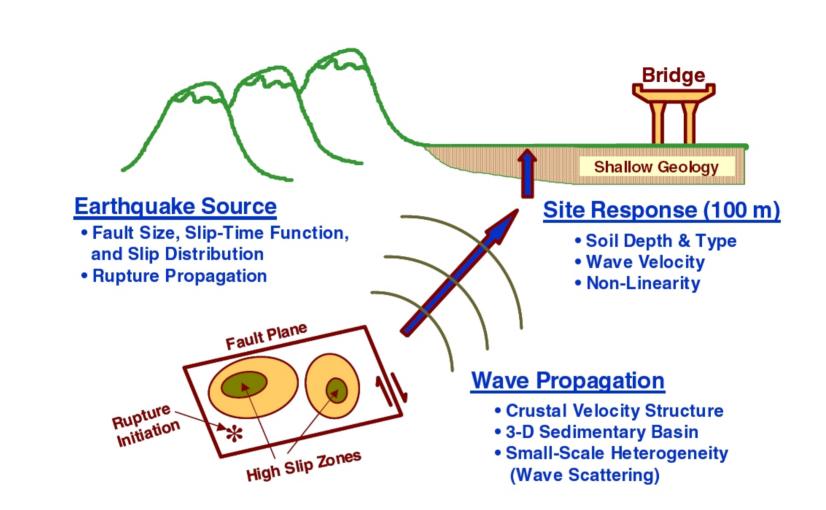
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## LIQUEFACTION

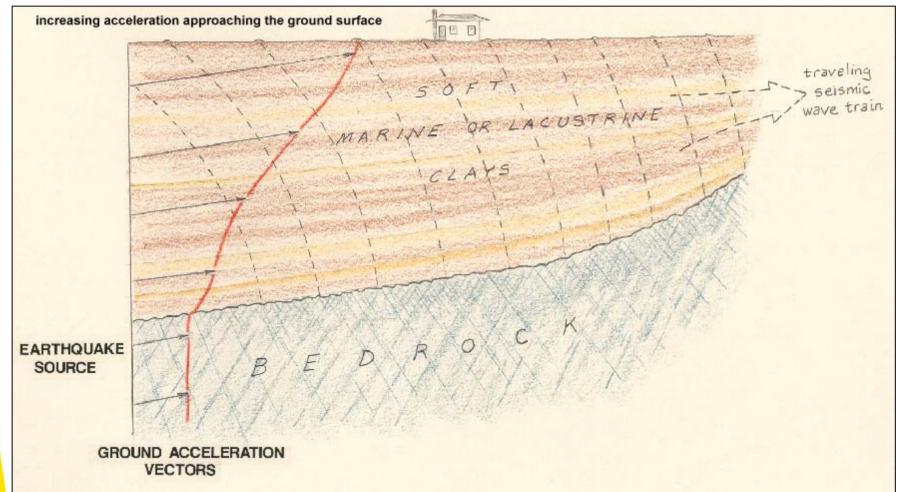
- Bridge failures during April 1991 M7.5 Costa Rica earthquake
- Though supported on steel and concrete piles respectively, these bridges both failed due to liquefaction of foundation materials, which tilted the piles



The type, depth and size of earthquake combine with geophysical properties of the underlying geology to affect seismic site response

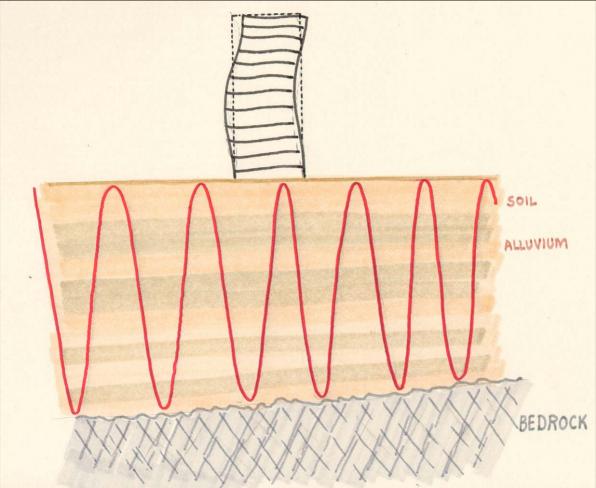
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## WHAT IS SITE RESPONSE ?



Site response is used to describe the fundamental period of vibration generated by a typical earthquake at any particular site. If soft unconsolidated sediments overlie resistant bedrock an impedance contrast develops at this boundary which causes incoming seismic energy to be absorbed at a rate faster than it can be transferred through the upper layers, causing significant amplification of ground motions.

### SITE RESPONSE VERSUS STRUCTURAL RESPONSE



The fundamental period of vibration of any structure depends on its design and construction details. If the site period and structural period converge, a resonant frequency results which may be an order of magnitude greater than the natural site period, and the structure will be severely damaged or UMR destroyed. LENGTHENING of SEISMIC WAVE TRAIN with DISTANCE from SOURCE

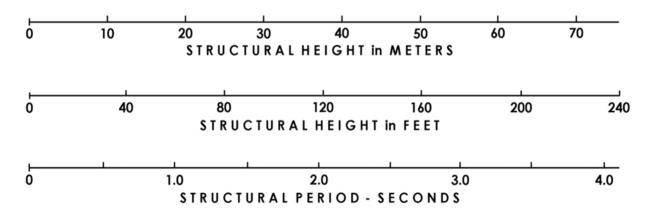


EARTHQUAKE SOURCE

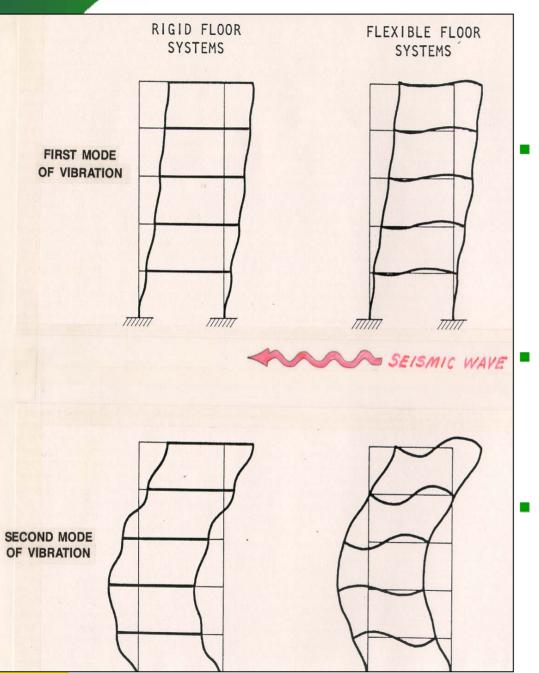
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NEAR FIELD MOTION ~ 0.3 to 0.5 seconds LONG PERIOD MOTION > 1.0 seconds

FUNDAMENTAL PERIOD vs STRUCTURE HEIGHT



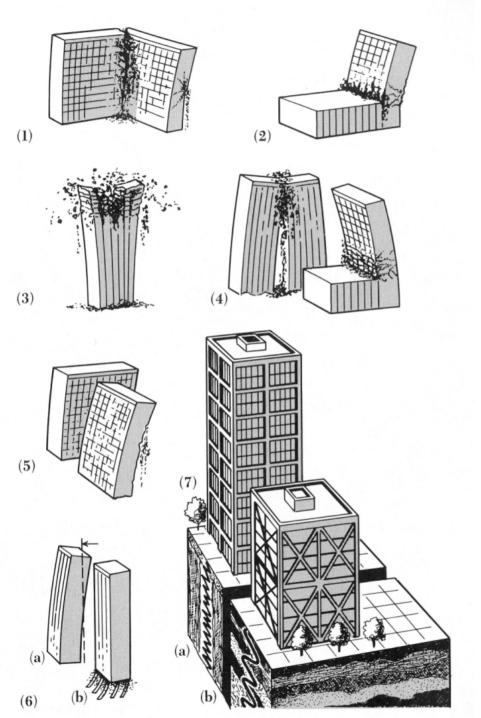
 Long period motions (T > 1.0 second) of great import when evaluating structures > 160 km from the quake hypocenter



## MODES OF VIBRATION

- All structures posses fundamental modes of vibration which depend on their skeletal make-up: including material type, shear panels, connections, span distances and symmetry.
- This fundamental mode is known as the "first mode of vibration" and it generally controls the seismic design of most symmetrical structures.
- Secondary modes of vibration become increasingly important in complex structures with asymmetrical form or stiffness, or structures with damaged frames.

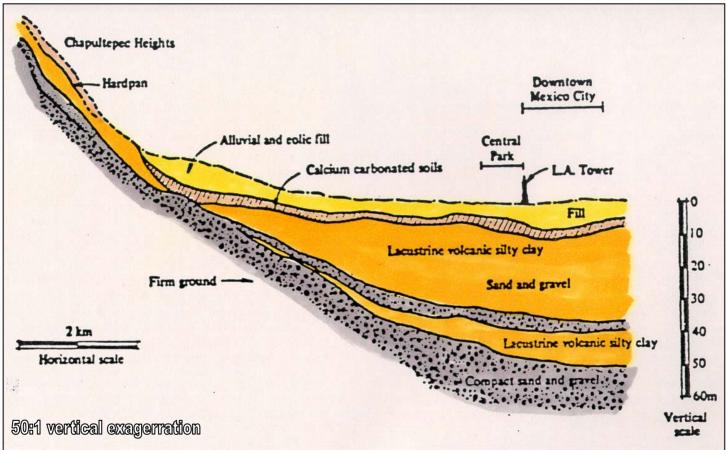




## OUT-OF-PHASE MOTION

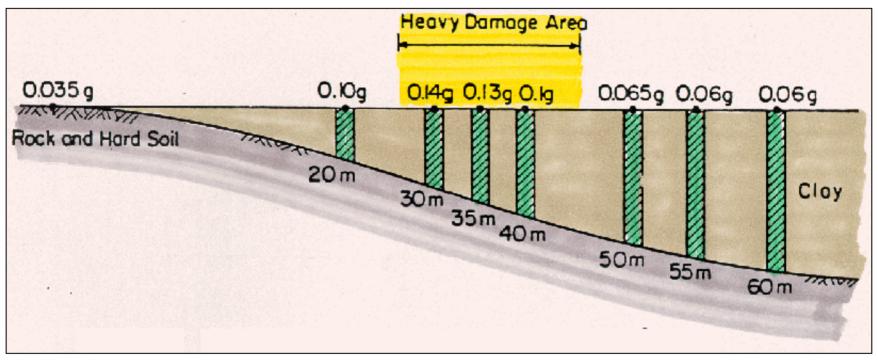
**Adjacent structures** can react differently to seismic excitation, depending on focal aspects of incoming energy, long period motion, site amplification, and degrading structural response as frames become damaged

## SOFT SEDIMENTS UNDERLYING MEXICO CITY



Generalized geologic cross section of the southern margins of the lacustrine basin underlying Mexico City. The lacustrine sediments were covered with fill as the city developed. These soft materials amplified the incoming seismic wave train from a M.8.1 earthquake located about 350 km from Mexico City!

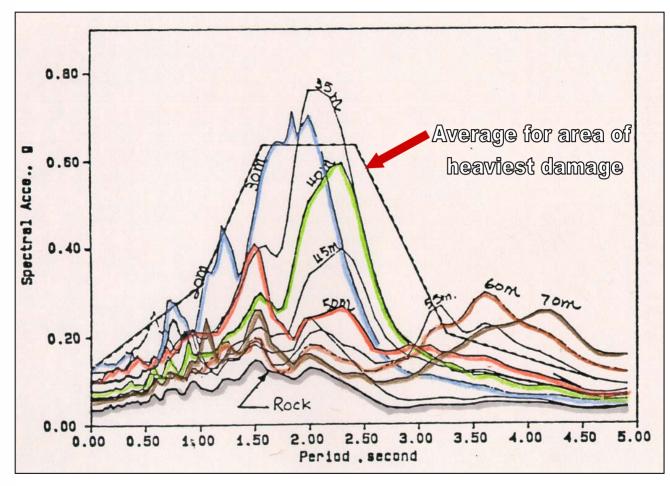
### ZONE OF HEAVIEST DAMAGE DURING 1985 MEXICO CITY EARTHQUAKE



Computed distribution of peak ground surface accelerations for typical soil profiles in Mexico City, bounding the zone that experienced severe damage during the 1985 M. 8.1 Michoacan earthquake. The earthquake shaking lasted close to 3 minutes. More than 500 buildings within the highlighted zone were severely damaged and 100 buildings between 6 and 22 stories high actually collapsed; killing 9,500, injuring 30,000 and leaving 100,000 homeless.

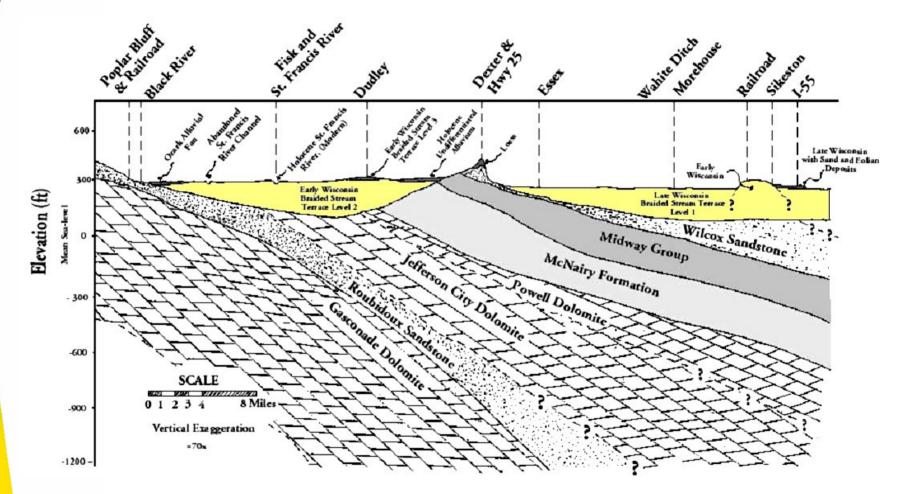


### VARIANCE OF RESPONSE SPECTRA WITH SEDIMENT THICKNESS IN MEXICO CITY



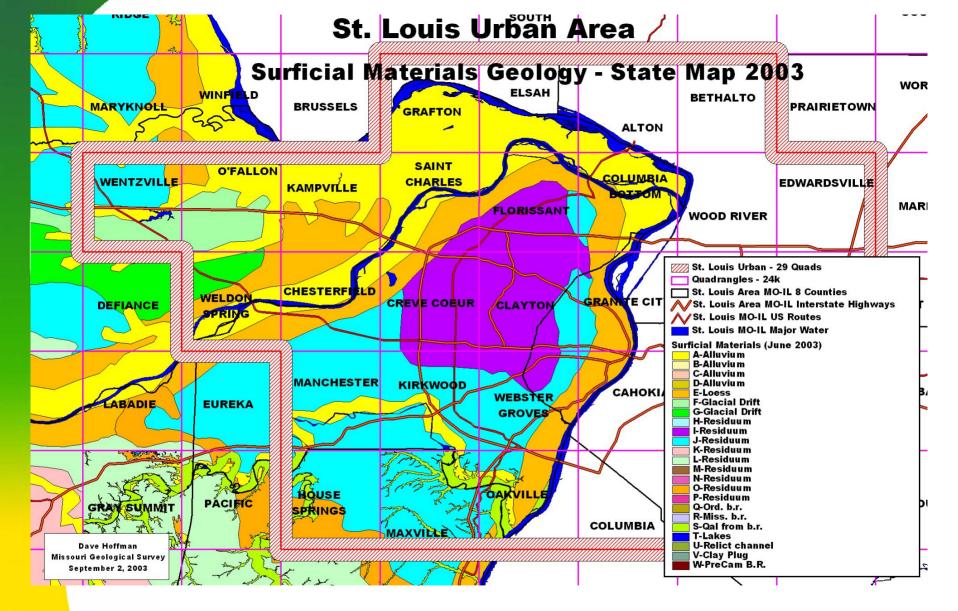
Response spectra calculated for different thicknesses of soft sediments in southern Mexico City, between downtown and Chapultepec Heights. Note impact of 30 to 45 m thickness.

## **Geology Northern Mississippi Embayment**



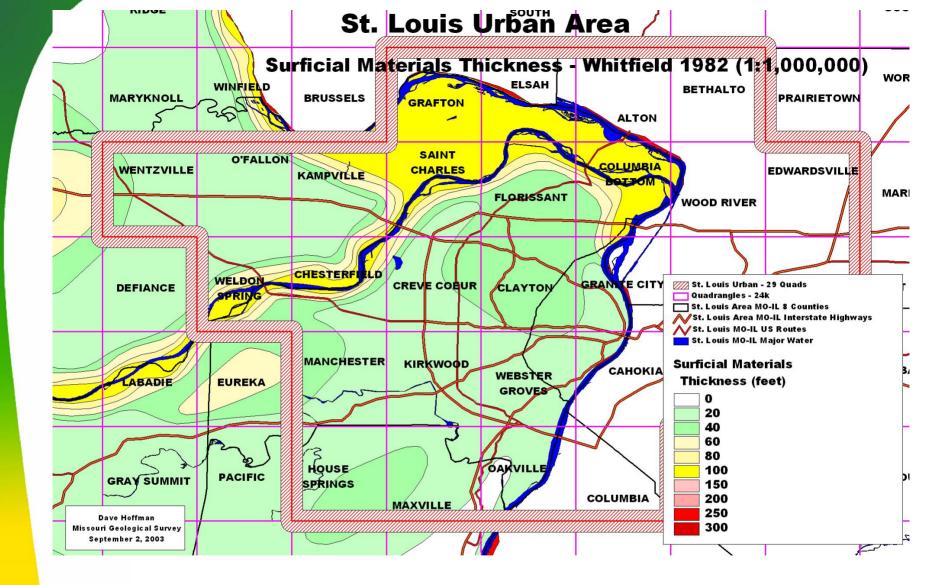
Impedance contrasts within the Wisconsin age river channels (yellow) likely pose the greatest seismic threat to structures in the Midwest.



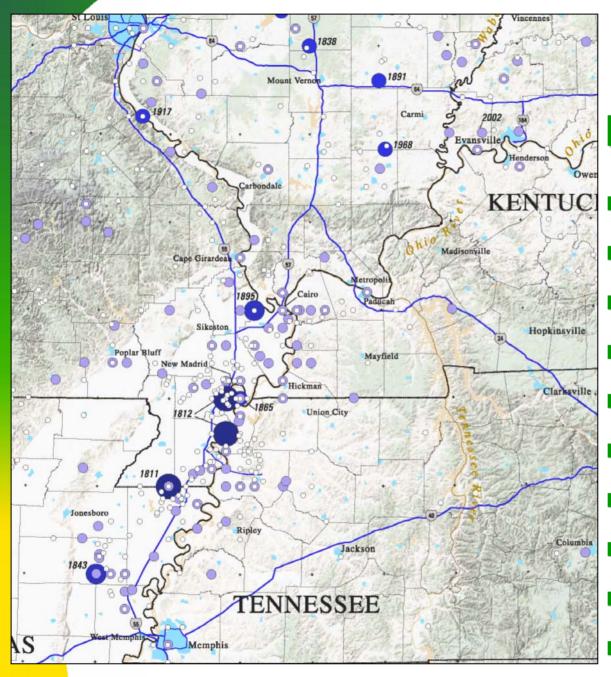


Geologic map of surficial materials in St.
 Louis area, west of Mississippi River

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 Depth to bedrock varies between 0 and about 100 feet in St. Louis, west of the Mississippi River



WHAT IS THE DESIGN **EARTHQUAKE?** >M7.5 in ~550 >M7.5 in ~900 >M7.5 in ~1450 M7.5+ in 1811 M8.0 in 1812 M6.3 in 1843 M6.6 in 1895 M5.4 in 1968 M5.0 in 1987 M4.6 in 2002



## **Recurrence Intervals for New Madrid Earthquake Events\***

| Magnitude | Recurrence Interval |
|-----------|---------------------|
| 4.0       | 14 Months           |
| 5.0       | 10 – 12 Years       |
| 6.0       | 70 – 90 Years       |
| 7.0       | 254 – 500 Years     |
| 8.0       | 550 – 1200 Years    |

\* based on existing data; always subject to update and revision

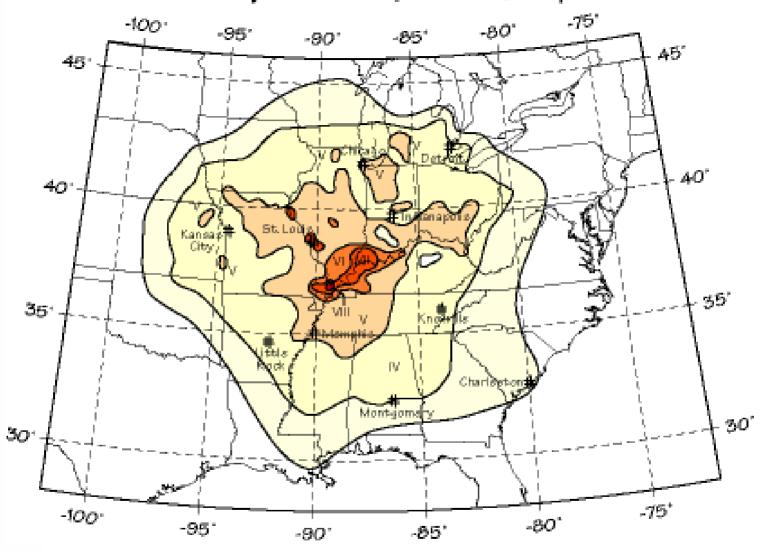


# **MOST LIKELY QUAKE**

- In our lifetimes, the most likely earthquake to impact St. Louis would be something similar to the Magnitude 6.6 Charleston, MO quake of 1895, which has a recurrence frequency of 70+/- 15 years (overdue since 1980).
- It could emanate from either the New Madrid Zone, the Wabash Valley Fault Zone, or from south central Illinois



#### Earthquake Shaking Intensity Map



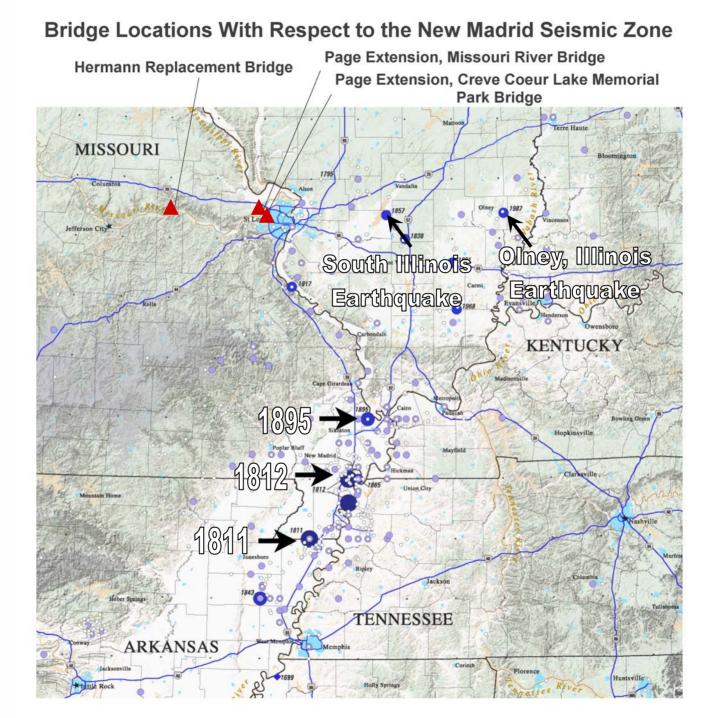
1895 M6.6 Charleston, MO earthquake



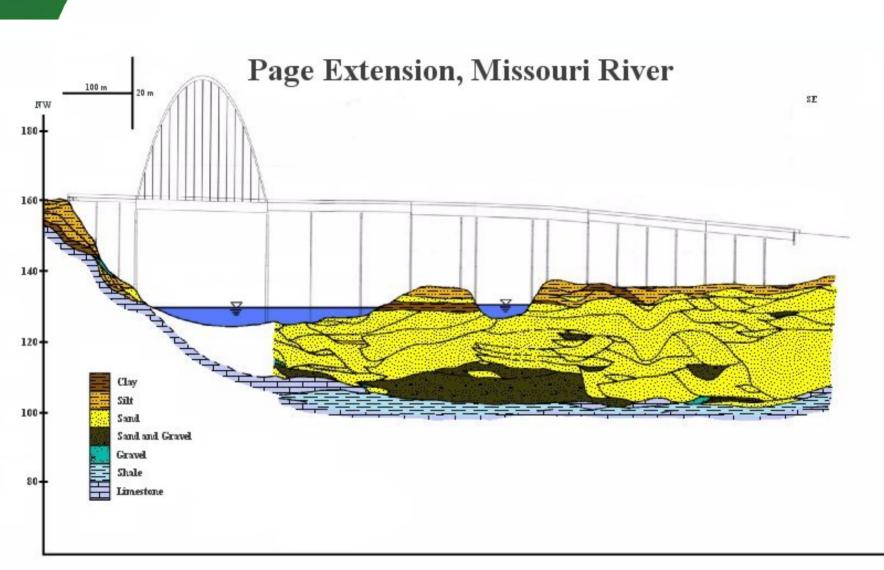
# **SCREENING ANALYSES**

- A series of site response evaluations were made on the Page Avenue Extension bridges in western St. Louis, located:
- About 115 km from the south central Illinois area which spawned the 1857 M 5.1 1857 quake
- about 215 km from the 1895 M 6.6
  Charleston, MO quake in the New Madrid Seismic Zone.
- About 223 km from the location of the 1987 M 5.0 Olney, IL quake in the Wabash Valley Fault Zone



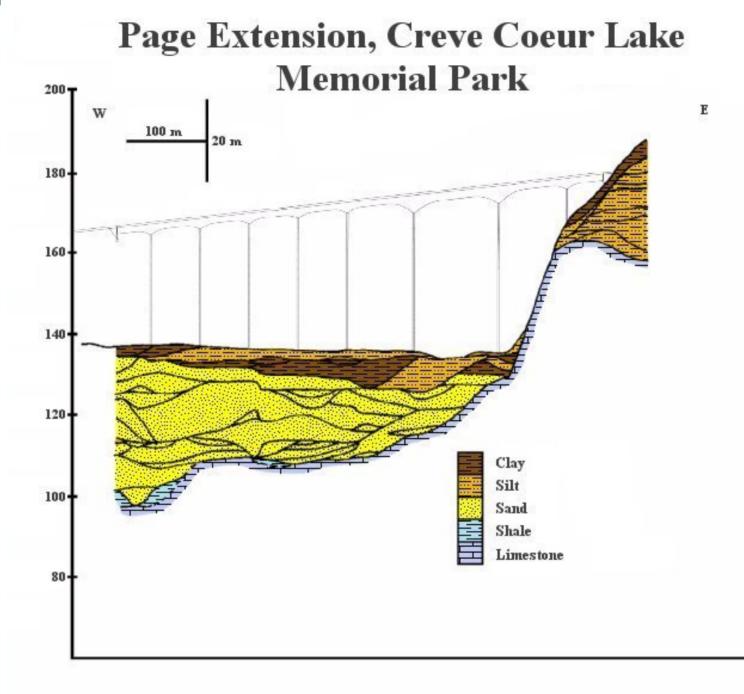






## The Missouri River channel filled with up to 31 m of unconsolidated alluvium, mostly sands

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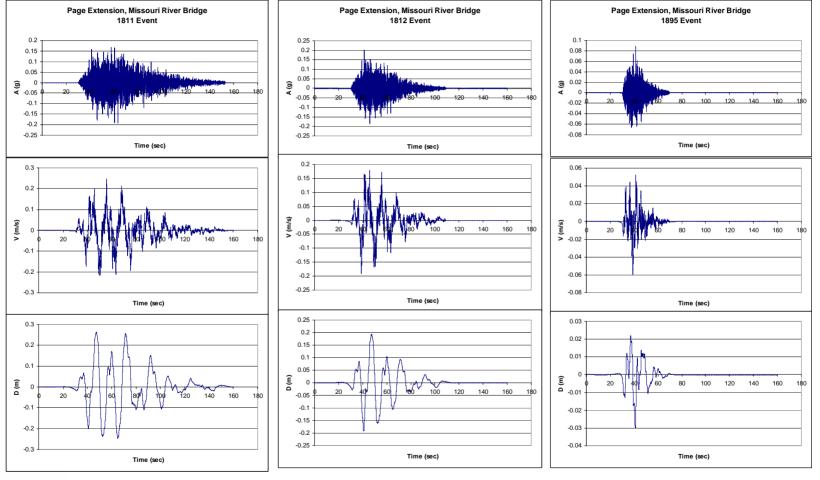


## ARTIFICIAL TIME HISTORIES FOR WERE GENERATED FOR THREE HISTORIC EVENTS IN THE NEW MADRID SEISMIC ZONE:

- If Dec 1811 M<sub>s</sub>8.6 = M 7.7 event, at distance of 312 km
- 7 Feb 1812 M<sub>s</sub> 8.0 = M 7.5 event, at distance of 261 km
- 31 Oct 1895 M<sub>s</sub> 6.8 = M 6.6 event, at distance of 230 km



## Page Ave. Bridge Artificial Time Histories for Quakes in NMSZ



1811

### 1812

### 1895



312.5 km distance

261.4 km distance

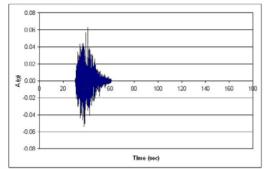
230.5 km distance

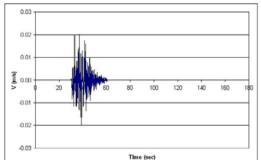
ARTIFICIAL TIME HISTORIES WERE ALSO GENERATED FOR A SERIES OF LIKELY EARTHQUAKES EMANATING FROM SOUTH CENTRAL ILLINOIS AND THE WABASH VALLEY FAULT ZONE:

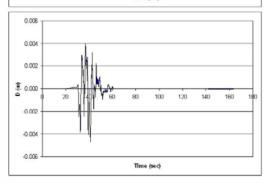
- M 6.0 and 6. 8 quakes at 115 km distance in south central Illinois (site of 1857 quake)
  - M 6.0 and 6.8 quakes at 223 km distance in Olney, Illinois (site of 1987 quake)

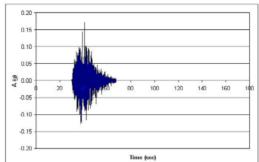


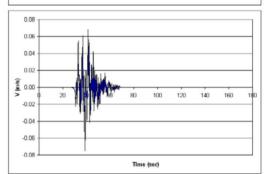
## Page Ave. Bridge Artificial Time Histories for Quakes in Illinois

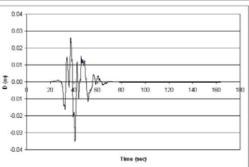












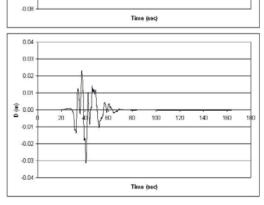


0.04 9 0.02 9 0.00 20 100 120 140 160 -0.02 -0.04 0.06 -0.08 Time (sec) 0.09 0.06 0.04 0.02 (s) 10.00 140 -0.02 -0.04

0.08

0.06

JO 06



Magnitude 6.8 223 km distance

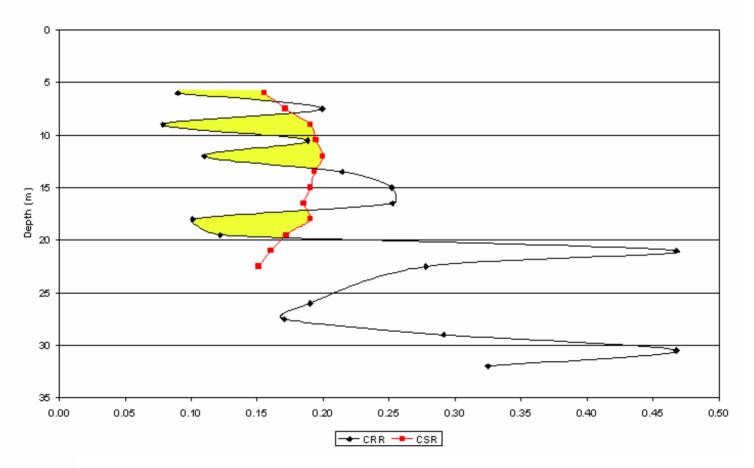
#### Magnitude 6 115 km distance

115 km distance



### Page Ave. Extension Missouri River Bridge M 7.3 New Madrid Seismic Zone event Cyclic Stress Ratio vs. Cyclic Resistance Ratio

Page Extension, Missouri River Bridge Boring B2-41 1811 Event

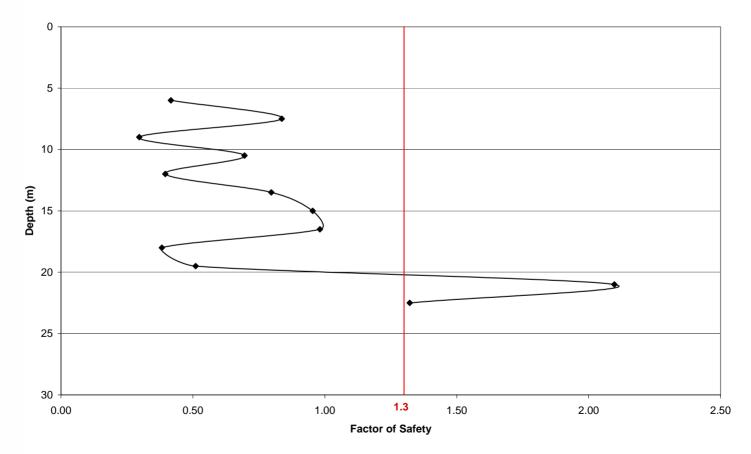


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1811 M8<sub>s</sub>.6

## Page Ave. Missouri River Bridge Liquefaction Factor of Safety 1812 M 7.5 NMSZ event

Page Extension, Missouri River Bridge Boring B2-41 Factor of Safety

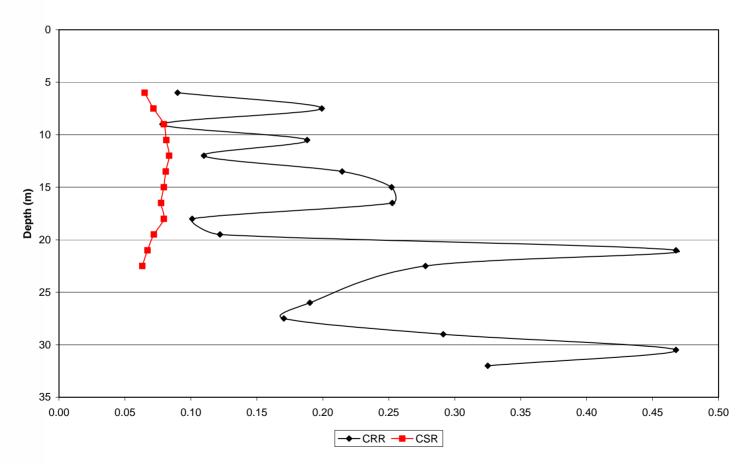




# Page Ave. M 6.6 NMSZ event

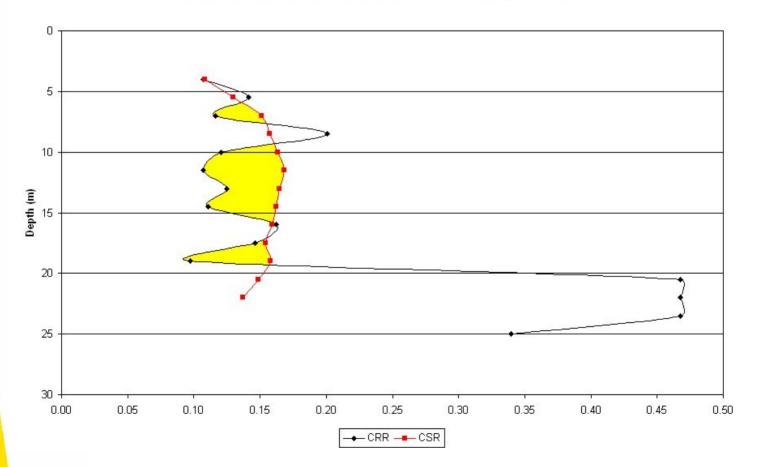
### **Cyclic Stress Ratio vs. Cyclic Resistance Ratio**

Page Extension, Missouri River Bridge Boring B2-41 1895 Event



## Page Ave. M 6.8 SCI event Cyclic Stress Ratio vs. Cyclic Resistance Ratio

Page Extension, Missouri River Bridge Boring B2-20 1857 Earthquake Location, Southern Illinois, 6.8 Magnitude, 115 km

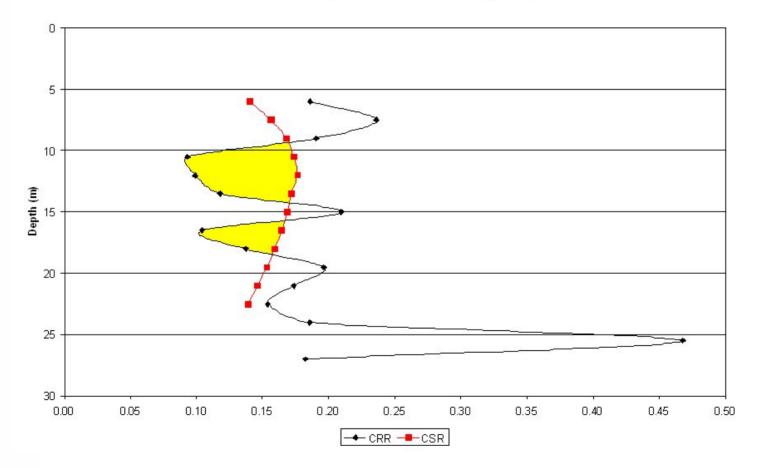


M<sub>s</sub>6.8 – 115 km distance



### **Creve Coeur Bridge M 6.8 SCI event Cyclic Stress Ratio vs. Cyclic Resistance Ratio**

Page Extension, Creve Coeur Lake Memorial Park Bridge Boring B2-61 1857 Earthquake Location, Southern Illinois, 6.8 Magnitude, 112.5 km

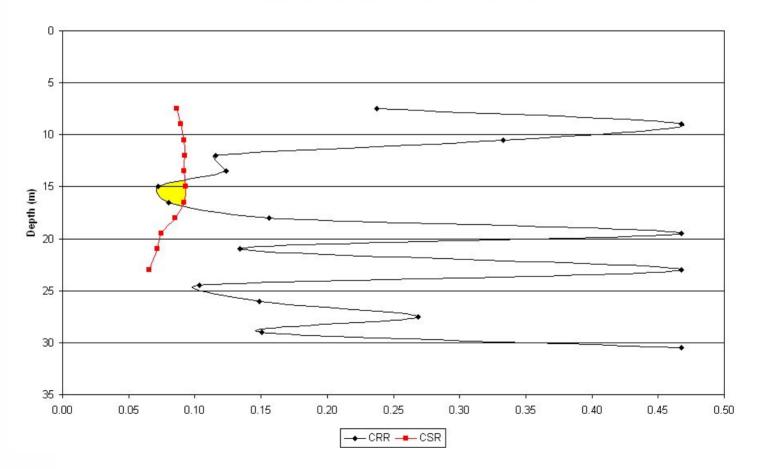


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M 6.8 – 112.5 km distance

## **Page Ave. M 6.8 WVFZ event** Cyclic Stress Ratio vs. Cyclic Resistance Ratio

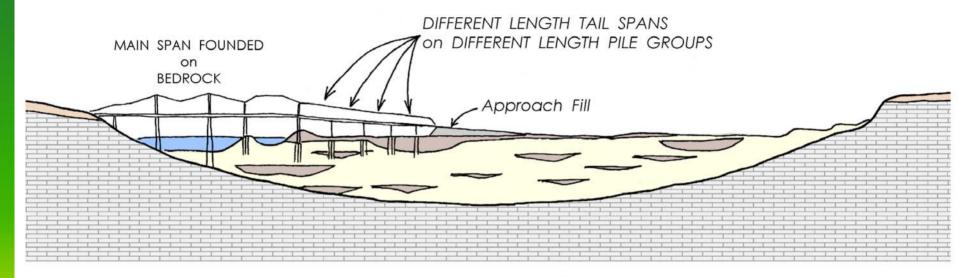
Page Extension, Missouri River Bridge Boring B2-23 1987 Earthquake Location, Olney - Illinois, 6.8 Magnitude, 223 km



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M<sub>s</sub>6.8 – 223 km distance

#### ELEMENTS of a TYPICAL CHANNEL CROSSING



- Asymmetric channel section; Missouri river on far south side of parabolic shaped channel
- Main spans supported on stiff caissons to rock
- Tail spans supported on pile groups of differing length
- Soft pockets on old oxbows can be problematic

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Widespread liquefaction and lateral spreads likely near channels

## **1D Seismic Site Response Equivalent Linear Approach**



1-D Wave Propagation Analysis Program for Geotechnical Site Response Analysis of Deep Soil Deposits

#### Main Features Include:

a) 1-D non-linear time domain wave propagation analysis method

b) 1-D equivalent linear frequency domain analysis method

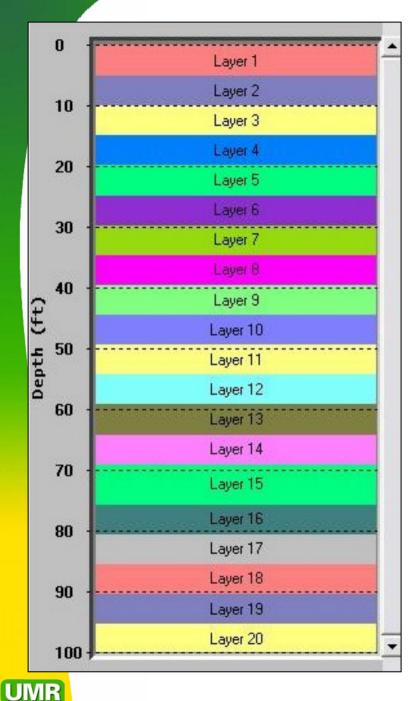
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Sponsored in part by project GT-3 Mid-America Earthquake Center NSF Grant EERC-9701785:

Developed by: Youssef Hashash and Duhee Park User Interface: Daniel Turner Help Manual: David Asfar

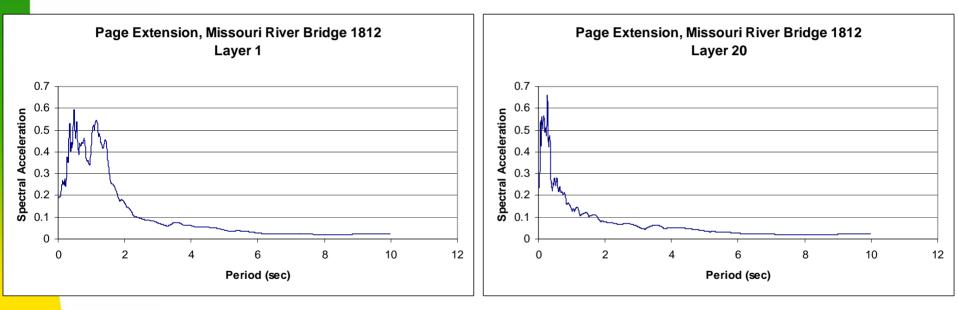
For future updates check staff.uiuc.edu/~hashash or contact hashash@uiuc.edu





**Soil Parameter Input Interface** using **DEEPSOIL 1-D wave** propagation analysis

### Page Ave. Missouri River Bridge M8.0 1812 NMSZ Event

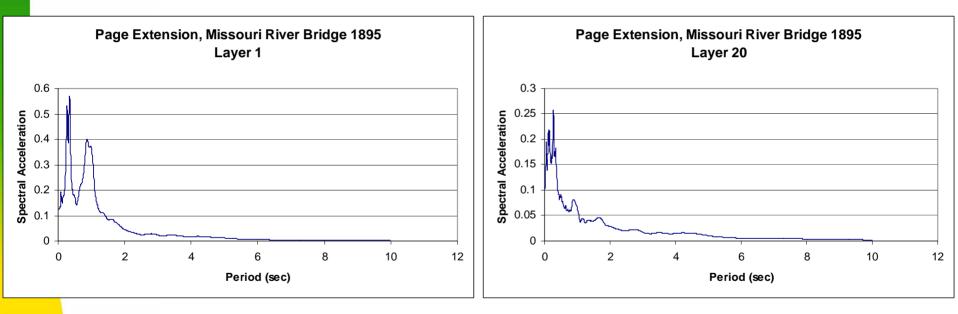


At ground surface

#### At bedrock interface



## Page Ave. Missouri River Bridge M6.6 1895 NMSZ Event

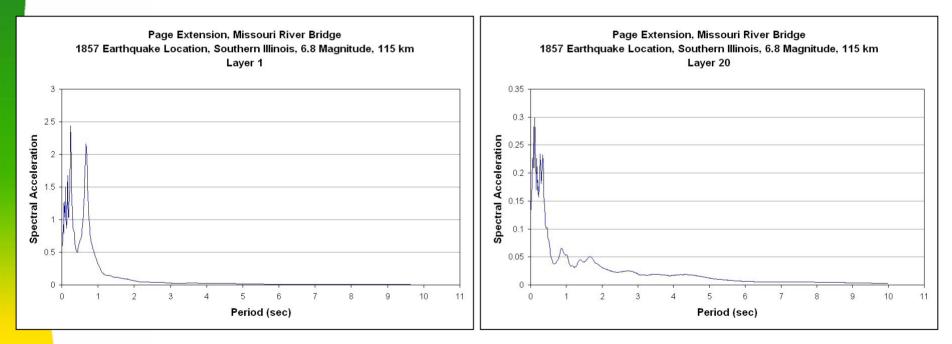


At ground surface Increases to 0.58 g

At bedrock interface  $a_{max} = 0.22g$ 



### Page Ave. Missouri River Bridge M6.8 115 km distance

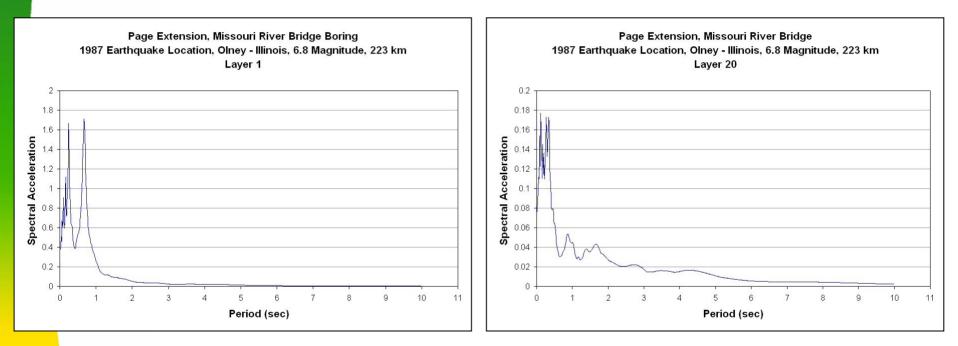


#### At ground surface

#### At bedrock interface



### Page Ave. Missouri River Bridge M6.8 223 km distance

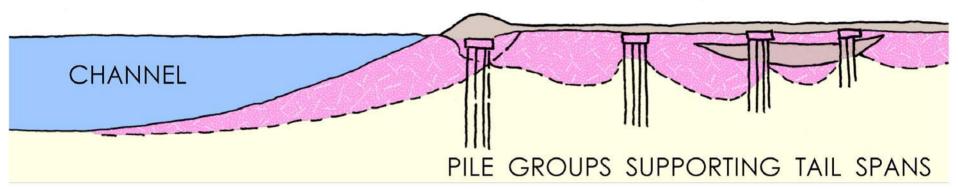


#### At ground surface

#### At bedrock interface



### ZONES COMMONLY SUSCEPTIBLE to LIQUEFACTION



- Simply supported tail spans probably most vulnerable component of long span highway bridges
- Site amplification causes long period motions to peak between 0.75 and 1.5 seconds; same period as bridges
- We can expect liquefaction of foundations (areas shown in pink)



## CONCLUSIONS

- Significant site amplification likely in deep valley fills, such as Mississippi and Missouri River channels
- Widespread liquefaction likely in Magnitude 6.6 or greater events at great range (155 miles/250 km)
- Liquefaction likely to be severe (deep) and continuous in Magnitude 7.5+ events





# This lecture will be posted online for easy downloading at:

## www.umr/edu/~rogersda/nmsz

